AMERICAN GEOTECH, INC.

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Mr. Anthony Koller Remedial Project Manager EPA, Region III 841 Chestnut Building Philadelphia, PA 19107 JUN 3 O 1994

June 21 1994

PA. Remodial Response Section

SUBJECT: EPA SUPERFUND SITE #3-HW-21 BERKS LANDFILL

REVIEW COMMENTS ON GOLDER'S DRAFT REMEDIAL INVESTIGATION REPORT SUPPLEMENT TO MY MARCH 11, 1994 LETTER

Dear Mr. Koller:

On behalf of the Concerned Citizens of Western Berks County, I have reviewed Golder's Draft Remedial Investigation Report dated March 1994. I have the following comments:

1. Existing Landfill Cap Condition - Golder's findings in their Executive Summary indicate below:

- The existing cap on the eastern landfill was constructed with a relatively good degree of quality control regarding thickness, compaction, and grading.

- The existing cap on the western landfill, while exhibiting lower cap thicknesses and field densities than the eastern landfill, also appears to have been compacted and graded during closure and has heavily vegetated side slopes.

- Overall, the existing cap materials are generally of the quality and character appropriate for landfill caps and are performing many of the functions for which they were designed.

However, our findings do not agree with Golder's. We have summarized Golder's field and laboratory test results in Table 1 and Figure 1 and have the following conclusions:

- The actual cap thicknesses vary from less than 2" to 36". More than 60% of both landfill areas have the thickness less than the specification (2').

- The capping materials consist of silty sand, fine to medium sand, silty gravel, clayey sand, or silty clay. More than half the landfill caps do not contain clayey materials.

- Out of 8 field density tests (73.6% to 100.9%), 5 of the tests were below 90% of the standard proctor density. Golder's acceptance criteria were based on average value and are totally unacceptable.

- The permeability tests were performed based on the recompacted samples. The permeability value is dependent upon the material type, compaction effort, natural moisture content, and other factors. The test quantity and test method are not representative. To verify the permeability of the landfill cap, the field permeability or permeability in laboratory on relatively undisturbed samples using Shelby tubes should be performed.

2. Geology and Hydrogeology - Information obtained from Borings G-4, G-5, and G-13 indicates that the upper layer of the diabase has hydraulic conductivities similar to fractured sandstone. With this limited information, it is inappropriate to conclude at this time that the diabase encircles the site in a saucer-like configuration, providing a physical and hydraulic barrier. If two flow systems; a shallow water table aquifer and a deeper semi-confined flow system exist under the existing landfills, additional information should be provided:

- Additional piezometers at different elevations inside the diabase and at different locations should be installed to confirm the actual potentiometric surface of the deeper aquifer, if any. Water samples from the deeper aquifer should also be collected and tested to include in this RI study.

- We believe that additional borings and piezometers may be necessary to be drilled and in-

stalled along the Cacoosing Creek tributary at the Northwest of the Site.

3. Groundwater - Please review my March 11, 1994 letter.

- The upgradient or background wells are too close to the landfills and may not be adequate to be considered as background wells.

- Golder only evaluated two sets of the test results for the on-site wells (9/3/92 and 7/93). Only 4 sets (9/92,6/93, 11/93, 1/94) of the test results for the residential wells were available for evaluation.

- Since some on-site monitoring wells were decommissioned and replaced at a significant distance from the original well locations, the replaced wells may not be representative.

- Chemicals detected by NUS/EPA since 1985 in the on-site or in the residential wells, should also be considered in this RI study, since the new monitoring wells may not be intercepting the waste on site due to the size of the site and also due to the inadequacy of the replacement of some decommissioned monitoring wells.

Should you have any questions concerning the above comments, please don't hesitate to call me or to set up a meeting to discuss them.

Very truly yours,

Kin Y C Chung, Ph.D., P.E. Consultant for CCWBC

Encl.

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TABLE 1 -ANALYSIS OF LANDFILL CAP QUALITY

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	CAP THICKNES	-	18-20(NR)	24(NH)	12(NE)	24(RE)	ç,	٧ ٧	24	8	24			30(NR)	27(NH)	<u>ء</u>	,	5 2	88	2:	7.	m (SO(NE)	36(NH)	18(NH)	200			۹ و	77	₹					
	PERMEABILITY			•		•	•		•	•	•			•	•	•	•	•	•	•	•	•		4.ZE-08 30(NR	() ()	6.15.07	•		1.ZE-00	•	•						
	PERCENT PE COMPACTION		•	•	•	•	•	•		•			•	•	•	•	•	•	•	•	•	•		9.0E+01	•	8.9E+01	•	. ;	1.0E+02	•	•						
	FIELD PE DRY CC DENSITY		97.8	88.3	114.1	97.1		101.9	99.2	114.8	101.3	7.101	<u>5</u>	102.8	100.7	94.9		105.7	108.7	104.5	102.6	•		89.8	103.9	87	87.2	•	103.1	•		96.2					
			•	•	•	•		•	•	•	•		•		•	•	•				•		•	111.5		109.2	•	•	102.2	•		•	LASSES	ESTERN	-	• •	4
	OPTIMUM MAXIMUM MOISTURE DRY CONTENT DENSITY		•	•	•	•	•	•		•	•	•	•	•	•	•	•	•	•	•	•		•	17	•	14.1	•	•	17.2	•	•	•	DISTRIBUTION OF SOIL CLASSES	EASTERN WESTERN	- σ	~	-
	OROUP SYMBOL		ರ	ರ	သွ	ပ္တ	•	•	S	သင	သင	ರ	သ	ರ	ರ :	¥	• ;	ပ္တ	ည	ပ္	ပ္ပ	¥	X	ರ	ರ	₩D ·	Z S	ပ္ပ	SK	ΣΘ	S.	S	DISTRIBUTION	ASS	ದ ೪	물	718
	MODIFICATIONS GROUP GRO MANTE SYM		SANDY LEAN CLAY	SANDY LEAN CLAY	CLAYEY SAND W/ GRAVEL	CLAYEY SAND	.•	•	SILTY SAND	CLAYEY SAND	CLAYEY SAND	SANDY LEAN CLAY	CLAYEY SAND	SANDY LEAN CLAY	SANDY LEAN CLAY	SANDY SILT	•	CLAYEY SAND W/ GRAVEL	CLAYEY SAND	SILTY GRAVEL W/ SAND	CLAYEY SAND		뚨	SANDY LEAN CLAY.	SANDY LEAN CLAY	SILTY GRAVEL W/ SAND	SHLTY SAND W/ GRAVEL	CLAYEY SAND W/ GRAVEL	SILTY SAND	SILTY GRAVEL W/ SAND	SILTY SAND W/ GRAVEL	SANDY SILT					
4	GROUP		SANDY LEAN CLAY	SANDY LEAN CLAY	CLAYEY SAND W/ GRAVEL	CLAYEY SAND	•	•	CLAYEY SAND	SHLTY SAND	CLAYEY SAND	SANDY LEAN CLAY	CLAYEY SAND	SANDY LEAN CLAY	SANDY LEAN CLAY	SANDY SILT	•	CLAYEY SAND W/ GRAVEL	CLAYEY SAND	SILTY GRAVEL W/ SAND	CLAYEY SAND		CLAYEY GRAVELWITH SAND*	SANDY LEAN CLAY	SANDY LEAN CLAY	SHITY GRAVEL W/ SAND	SILTY SAND W/ GRAVEL	CLAYEY SAND W/ GRAVEL	SILTY SAND	SILTY GRAVEL W/ SAND	SILTY SAND W/ GRAVEL	SANDY SILT					•
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TABLE 1 - ANALYSIS OF LANDFILL CAP QUALITY (CONT)

200		OF A CTIC	MACTIC MARTICIT	ABOUR	MOONE	MODIFICATIONS	OPTIMUM MAXIMUM	AAXIMUM	FIELD	PERCENT	PERMEABILITY	
9	LIMIT	TIME	MDEX		GROUP	SYMBOL	MOISTURE CONTENT	DAY Density	DENSITY	COMPACTION		CAP THICKNES
PHASE 1A						-						
ELSSP-1	•	•	•		•	•	•	•	•	•	•	24
ELSSP-2	•		•	BROWN SILTY CLAY WITH	•	., •	· •		•	•	•	12
ELSSP-3		•	•	SOME GRAVEL C-F GRAVEL AND F-M SANDS	•	•		•	108.7	•	•	27
ELSSP-4	•	•	•	SOME FINES F-M SAND AND FINES,	ď	•	13.7	119.5	115.1	96.31799163	1.60005-07 14	7
ELSSP5	•	•	•	SOME GRAVEL SILTY CLAY, SURFICIAL SOIL	•	•	•		•	•	•	77
ELSSP-6	•	•	•	INC. MANY COBBLES FM SAND AND FINES,	33		17.7	108	79.5	79.5 73.61111111	8.50006-08 15	15
ELSSP-7	•	•	1	SOME F.C GRAVEL F.M SAND AND FINES,	ď	•	12	118.5	114.8	96.87763713	9.40006-08 6	•
8-45STB	•	•	•	SOME C-F GRAVEL SILTY TO ORGANIC CLAY,	•	٠.	•	•	• .	•	•	12
ELSSP9	. •	•	•	WITH COBBLES SILTY CLAY	•	•	•	•	•	•	•	15
ELSSP-10	•	•	•	SILTY CLAY/CLAYEY SILT,	•		•		•	•	•	23
ELSSP-11	•	•	•	COBBLES ON SURFACE SULTY CLAY WITH COBBLES	·•	•	•	•		•	•	>30
ELSSP-12	•	•	•	ON SURFACE SANDY SILT ABOVE POSS.	•	•	•	•	•	•	•	9>
ELSSP-13	•	•		ORGANIC MATTER SILTY CLAY W/ COBBLES	•	· •	•			•	•	31
ELSSP.14	•	•	•	ABOVE MOIST CLAYEY SILT	•	•	•	•		•	,	28
ELSSP-15	•	•	•	•	•		•	•.		•	•	96
ELSSP-18	•	•	•	CLAY TO CLAYEY SILT WITH		•	•	•	•	•	•	89
PEISSP-17			•	PLASTIC AND GLASS CLAYEY SILT WITH CORRES,	•	•	•	•		•	•	9
CTWISSP-1		•		RUBBER, WINE, AND MASTIC F-M SAND AND C-F GRAVEL,	•	•	14.2	113.2	84.2	74.38162544	0.000075 18	18
O WISSP-2	•	•	•	SOME FINES SULTY CLAY WITH ROCK	•	è		•	•		•	25
Owissp.3	,	•	•	SULTY CLAY WITH SMALL		•	•	•	•	•		28
COWLSSP-4	ı	•	•	FINES AND F-C SAND,	٠.	•	14.5	116.8	101.7	87.07191781	1.70006-07 14	4
WLSSPL5	đ	•	•	SULT WITH SMALL ROCK	•	•	•	. •		•	•	0-5
WLSSP-6	٠.	•	•	FRAUMENIS SILTY CLAY WITH SMALL ROCK FRAGMENTS		•	•		.•		•	8

